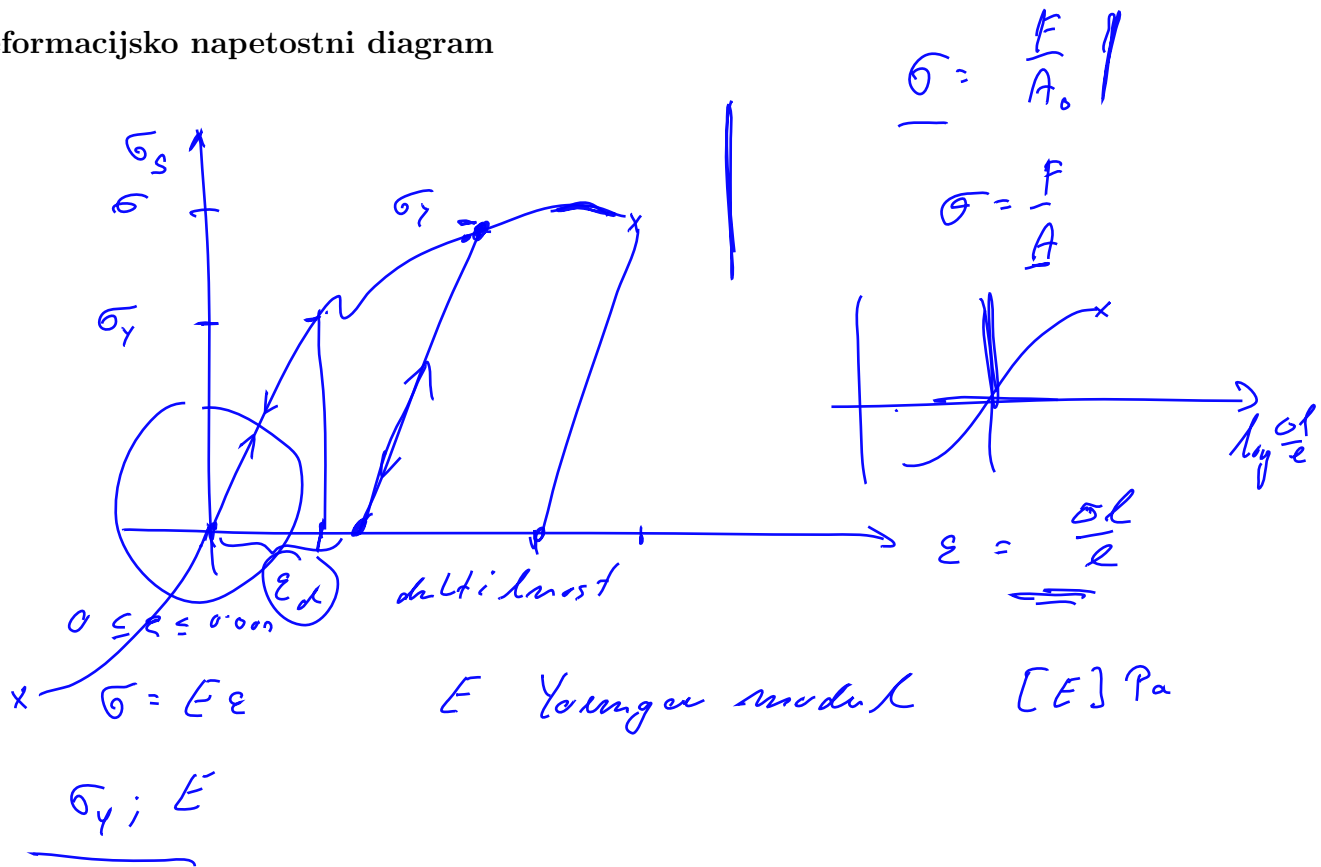


Deformacijsko napetostni diagram



Značilne točke in območja na deformacijsko napetostnem diagramu.

Tabela Youngovih modulov E , mej tečenj σ_Y in nateznih trdnosti σ_S .

Primer: določi dopustno deformacijo, da napetost ne preseže meje tečenja.

Baker $E = 120 \text{ GPa}$ $\sigma_Y = 60 \text{ MPa} = 60 \cdot 10^6 \text{ Pa}$
 $\text{GPa} = 10^9 \text{ Pa}$

$$\sigma = E \varepsilon$$

Kloekur zalom $\sigma_Y \geq \sigma = E \varepsilon$

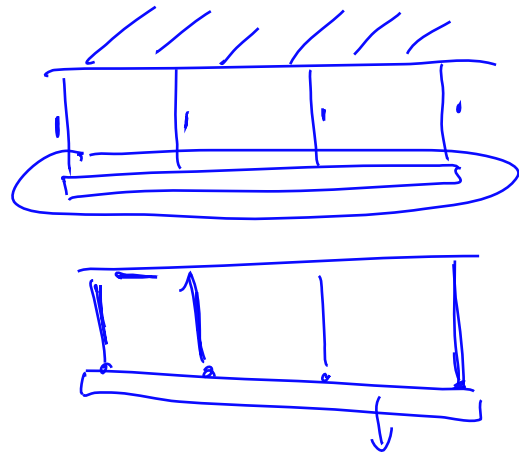
$$\Rightarrow \varepsilon < \frac{\sigma_Y}{E} = \frac{60 \text{ MPa}}{120 \text{ GPa}} = \frac{1}{2} \cdot 10^{-2}$$
$$= \boxed{0.0005}$$
$$\frac{F}{A} = E \left(\frac{\Delta l}{l} \right)$$

$$F = A\sigma$$

$$\underline{\underline{\sigma = E\varepsilon}}$$

Reševanje statično nedoločenih nalog

Primer: nosilec obešen na štiri žice.



Termoelastičnost

Primer: določi napetost palice med dvema togima stenama pri spremembi temperature za ΔT .

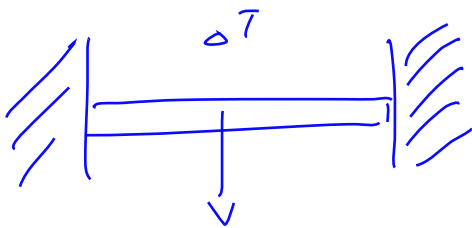
ε_T temperaturna deformacija

$$\underline{\underline{\varepsilon_T = \alpha \Delta T}}$$

$$\text{Al } \alpha = 23 \cdot 10^{-6} \frac{1}{\text{K}}$$

$$\text{Fe } \alpha = 118 \cdot 10^{-6} \text{K}^{-1}$$

$$\text{beton } \alpha = 12 \cdot 10^{-6} \text{K}^{-1}$$



$$\varepsilon = \varepsilon_T + \varepsilon_E = 0 \quad \sigma = E\varepsilon_E$$

$$0 = \alpha \Delta T + \frac{\sigma}{E} \Rightarrow \underline{\underline{\sigma = -\alpha E \Delta T}}$$

$$E = 69 \text{ GPa} ; \alpha = 231 \cdot 10^{-6} \text{K}^{-1}$$

$$\sigma = -231 \cdot 69 \cdot 10^9 \text{ Pa} \cdot 10^{-6} \text{K}^{-1} \cdot \Delta T = -1600 \cdot 10^3 \frac{\sigma}{\text{K}} \text{ Pa}$$

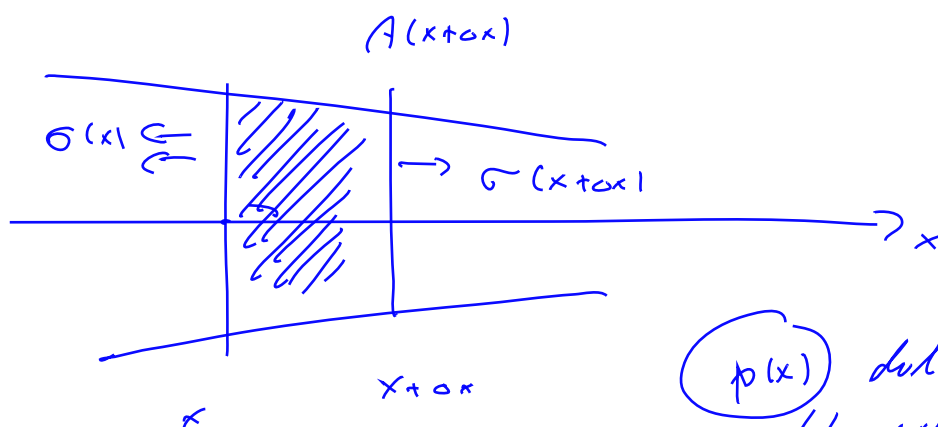
$$\sigma = -16 \cdot 10^6 \text{ Pa} \frac{\sigma}{\text{K}} = \underline{\underline{-16 \text{ MPa} \frac{\sigma}{\text{K}}}}$$

$$\sigma_y = 40 \text{ MPa}$$

$$16 \cdot \frac{\sigma}{\text{K}} < 40$$

$$\sigma T < \frac{40}{16} \text{ K} = \underline{\underline{25 \text{ K}}}$$

Ravnovesna enačba osne napetosti in deformacije



$p(x)$ dolžinska
gostota uteži
sila

$$\sigma(x+dx)A(x+dx) - \sigma(x)A(x) + \int_x^{x+dx} p(s)ds = 0 \quad \left| \frac{1}{dx} \lim_{dx \rightarrow 0} \right.$$

Ravnovesna enačba za osno napetost

$$\frac{d}{dx} (\sigma(x)A(x)) + p(x) = 0$$

$$\frac{\sigma(x+dx)A(x+dx) - \sigma(x)A(x)}{dx} + \left(\frac{1}{dx} \int_x^{x+dx} p(s)ds \right) = 0$$

$$\frac{d}{dx} (\sigma A) + p(x) = 0 \quad \sigma = E\varepsilon = E \frac{du}{dx}$$

Ravnovesna enačba za pomik

$$\frac{d}{dx} \left(AE \frac{du}{dx} \right) + p(x) = 0.$$

Sila teže

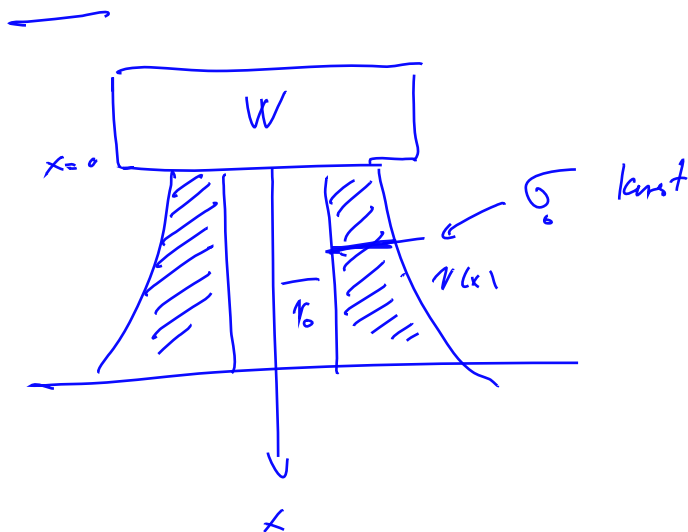
$$F_g = \rho g V = \rho g A L$$

$$V = A L$$

4

$$p = \rho g A$$

Primer: sila teže: dolžinska gostota je $p(x) = \rho(x)A(x)g$.



Primer: problem vodnega stolpa

$$-\sigma_0 \cdot A_0 = W$$

$$\sigma_0 = - \frac{W}{A_0}$$

kompresijska napetost

$$\frac{d}{dx}(\sigma A) + p = 0$$

$$p = \rho g A(x)$$

$$\frac{d}{dx}(\sigma_0 A(x)) + \rho g A(x) = 0 \Rightarrow \frac{dA}{dx} = - \frac{\rho g}{\sigma_0} A$$



$$\frac{dA}{dx} = \left(\frac{\rho g A_0}{W} \right) A ; \quad \left[\frac{dA}{dx} = k A \right]$$

$$\frac{de^x}{dx} = e^x$$

$$A = C e^{kx}$$

$$A(x=0) = C e^0 = C = A_0$$

$$A(x) = A_0 e^{kx}$$

$$A(x) = \pi (r^2 - r_0^2)$$

$$r^2 = r_0^2 + \frac{1}{\pi} A_0 e^{kx} = r_0^2 + \frac{1}{\pi} \frac{W}{\sigma_0} e^{\frac{\rho g W}{W \sigma_0} x} = r_0^2 + \frac{1}{\pi} \frac{W}{\sigma_0} \left(e^{\frac{\rho g}{\sigma_0} x} \right)$$

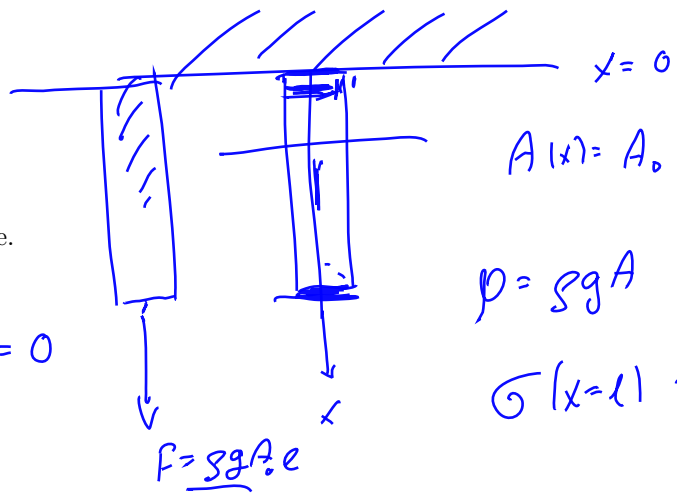
Debelina raste eksponentno

$$e^{\frac{\rho g}{\sigma_0} x}$$

$$\frac{\rho}{\sigma_0}$$

$$\sigma_0 = \rho \sigma_s$$

$\frac{8}{\sigma_s}$



Primer: deformacija palice zaradi lastne teže.

$$\frac{d}{dx} (A\sigma) + p(x) = 0$$

$$\sigma = E \frac{du}{dx}$$

$$A_0 E \frac{d^2 u}{dx^2} = -\rho g A_0 \Rightarrow \frac{d^2 u}{dx^2} = -\frac{\rho g}{E}$$

$$u = \frac{1}{2} \left(-\frac{\rho g}{E} \right) x^2 + C_1 x + C_2$$

$$\left. \begin{array}{l} u(x=0) = 0 \\ \frac{du}{dx}(x=l) = 0 \end{array} \right\} \rightarrow C_2 = 0$$

$$-\frac{\rho g}{E} x + C_1 = 0 \Rightarrow C_1 = \frac{\rho g}{E} l$$

$$u = \frac{\rho g}{E} \left(l - \frac{1}{2} x \right) x$$

$$u(x=l) = \frac{1}{2} \frac{\rho g}{E} l^2$$

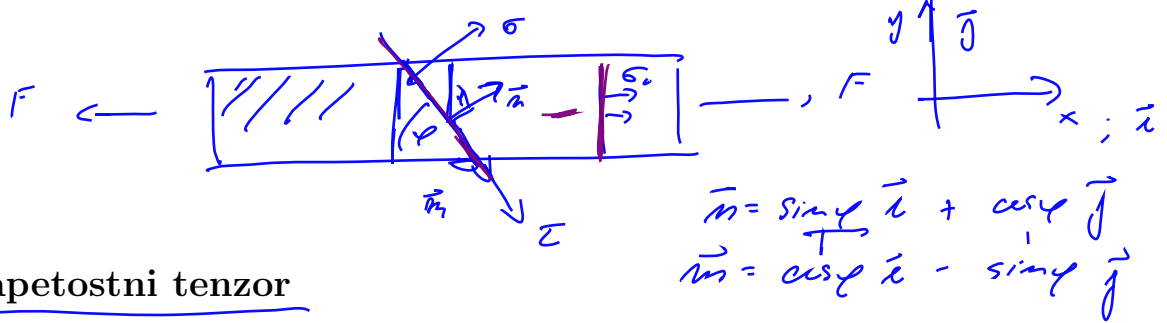
$$\frac{\Delta l}{l} = \frac{u(x=l)}{l} = \frac{1}{2} \frac{\rho g}{E} l$$

$$\sigma = \frac{F}{A_0} = \rho g l = E \epsilon \Rightarrow$$

$$\epsilon = \frac{\Delta l}{l} = \frac{\rho g l}{E}$$

$$\frac{du}{dx} = -\frac{\rho g}{E} x + \frac{\rho g}{E} l = \frac{\rho g}{E} (l - x)$$

deformacija ni enakomerna.



Napetostni tenzor

Poševni presek palice, stržna napetost, odvisnost od kota preseka.

$$A(\sigma \vec{n} + \tau \vec{m}) = F = A_0 \sigma_0 \vec{e} \quad A_0 = A \cdot \cos(\frac{\pi}{2} - \varphi)$$

$$\begin{cases} \sigma \sin \varphi + \tau \cos \varphi = \sigma_0 \\ \sigma \cos \varphi - \tau \sin \varphi = 0 \Rightarrow \tau = \sigma \frac{\cos \varphi}{\sin \varphi} \end{cases} \quad A_0 = A \sin \varphi$$

$$\sigma + \tau \frac{\cos \varphi}{\sin \varphi} = \sigma_0 \Rightarrow \sigma \left(1 + \frac{\cos^2 \varphi}{\sin^2 \varphi}\right) = \sigma_0 \Rightarrow \sigma \frac{1}{\sin^2 \varphi} = \sigma_0$$

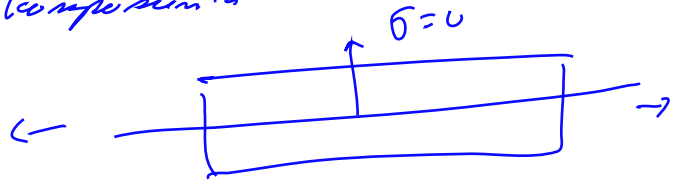
(Vektor napetosti $\vec{t} = \vec{t}(p, \vec{n})$ je odvisen od smeri prereza. Ta odvisnost je linearna. To pomeni, da obstaja tenzor napetosti \underline{t} tako, da je $\vec{t} = \underline{t} \vec{n}$.)

$$\begin{cases} \sigma = \sin^2 \varphi \sigma_0 = \frac{1}{2} (1 - \cos 2\varphi) \sigma_0 \\ \tau = \sin \varphi \cos \varphi \sigma_0 = \frac{1}{2} \sin 2\varphi \sigma_0 \end{cases}$$

$$\begin{aligned} \sin^2 \varphi + \cos^2 \varphi &= 1 \\ \cos^2 \varphi - \sin^2 \varphi &= \cos 2\varphi \end{aligned}$$

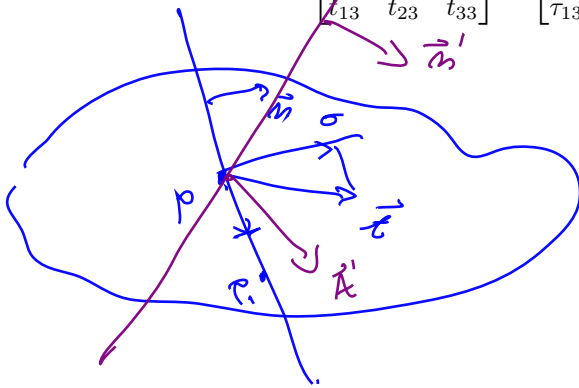
normalna komponenta

τ stržna komponenta



Matrični zapis tenzorja napetosti:

$$\underline{t} = \begin{bmatrix} t_{11} & t_{12} & t_{13} \\ t_{12} & t_{22} & t_{23} \\ t_{13} & t_{23} & t_{33} \end{bmatrix} = \begin{bmatrix} \sigma_1 & \tau_{12} & \tau_{13} \\ \tau_{12} & \sigma_2 & \tau_{23} \\ \tau_{13} & \tau_{23} & \sigma_3 \end{bmatrix}$$



ρ gostota povprečne sile

σ normalna napetost

$$t_n = \sigma = \underline{t} \cdot \vec{n} \quad \vec{t}_m = t_m \vec{e}_m$$

τ stržna napetost

Tenzor napetosti je simetričen in ima 6 neodvisnih komponent.

$$|\vec{t} - \vec{t}_m| = \tau$$

$$\vec{t} = \vec{t}_n + \vec{t}_s$$

$$\vec{t}_s = \vec{t} - \vec{t}_n \quad \text{vektor stržnih napetosti}$$

$$\vec{t} = \underline{t}(p, \vec{n})$$

\vec{t} je linearno odvisen od \vec{n}

$$\vec{t} = \underline{t}(p) \vec{n}$$

\underline{t} napetostni tenzor

$$\vec{\tau} = \tau_1 \vec{i} + \tau_2 \vec{j} + \tau_3 \vec{k} \quad \vec{m} = m_1 \vec{i} + m_2 \vec{j} + m_3 \vec{k}$$

$$\begin{bmatrix} \tau_1 \\ \tau_2 \\ \tau_3 \end{bmatrix} = \begin{bmatrix} t_{11} & t_{12} & t_{13} \\ t_{21} & t_{22} & t_{23} \\ t_{31} & t_{32} & t_{33} \end{bmatrix} \begin{bmatrix} m_1 \\ m_2 \\ m_3 \end{bmatrix} = \begin{bmatrix} t_{11}m_1 + t_{12}m_2 + t_{13}m_3 \\ - \\ - \end{bmatrix}$$

Normalna napetost, vektor normalne napetosti; strižna napetost, vektor strižne napetosti.

$$\tau_{ij} = \tau_{ji} \quad \underline{\tau} = \begin{bmatrix} t_{11} & t_{12} & t_{13} \\ t_{21} & t_{22} & t_{23} \\ t_{13} & t_{23} & t_{33} \end{bmatrix}$$

$$\vec{\tau} = \underline{\tau} \vec{m} \quad \tau_m = \vec{\tau} \cdot \vec{m} = \vec{m} \cdot \underline{\tau} \vec{m} = \begin{bmatrix} m_1 \\ m_2 \\ m_3 \end{bmatrix} \cdot \begin{bmatrix} t_{11} & t_{12} & t_{13} \\ \cdot & t_{22} & t_{23} \\ \cdot & \cdot & t_{33} \end{bmatrix} \begin{bmatrix} m_1 \\ m_2 \\ m_3 \end{bmatrix}$$

$$\tau_s = |\vec{\tau} - \tau_m \vec{m}| \quad ; \quad \vec{\tau}_m = \tau_m \vec{m} = (\vec{m} \cdot \underline{\tau} \vec{m}) \vec{m}$$

$$|\vec{m}| = 1 \quad m_1^2 + m_2^2 + m_3^2 = 1$$

Pomen komponent napetostnega tenzorja v danem KS:

- diagonalni elementi so enaki normalnim napetostim v koordinatnih smereh;
- izven diagonalni elementi so enaki projekciji vektorjev strižne napetosti na koordinatne osi.

$$\vec{m} = \vec{i} \quad \vec{\tau} = \underline{\tau} \vec{m} = \underline{\tau} \vec{i} = \begin{bmatrix} t_{11} & t_{12} & t_{13} \\ t_{12} & t_{22} & t_{23} \\ t_{13} & t_{23} & t_{33} \end{bmatrix} \begin{bmatrix} 1 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} t_{11} \\ t_{12} \\ t_{13} \end{bmatrix}$$

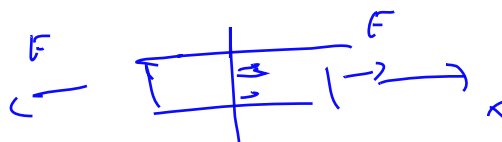
$$\vec{\tau}_1 = \begin{bmatrix} t_{11} \\ t_{12} \\ t_{13} \end{bmatrix} \quad \tau_m = \vec{m} \cdot \vec{\tau} = \begin{bmatrix} 1 \\ 0 \\ 0 \end{bmatrix} \cdot \begin{bmatrix} t_{11} \\ t_{12} \\ t_{13} \end{bmatrix} = t_{11}$$

$$\vec{\tau} = \vec{\tau}_m + \vec{\tau}_s \quad \vec{\tau}_m = \tau_m \vec{m} = t_{11} \begin{bmatrix} 1 \\ 0 \\ 0 \end{bmatrix}$$

$$\begin{bmatrix} t_{11} \\ t_{12} \\ t_{13} \end{bmatrix} = \begin{bmatrix} t_{11} \\ 0 \\ 0 \end{bmatrix} + \vec{\tau}_s \Rightarrow \vec{\tau}_s = \begin{bmatrix} 0 \\ t_{12} \\ t_{13} \end{bmatrix}$$

$$\underline{\tau} = \begin{bmatrix} t_{11} & t_{12} & t_{13} \\ & t_{22} & t_{23} \\ & & t_{33} \end{bmatrix} = \begin{bmatrix} \sigma_1 & \tau_{12} & \tau_{13} \\ \tau_{12} & \sigma_2 & \tau_{23} \\ \tau_{13} & \tau_{23} & \sigma_3 \end{bmatrix}$$

Osnovna napetostna stanja



- Enoosno napetostno stanje; izračun normalne in strižne napetosti.

$$\underline{\underline{\tau}} = \begin{bmatrix} \sigma & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$\vec{n} = \vec{e}_1 \quad \tau_n = \tau_{11} = \sigma$$

$$\vec{\tau}_s; = \underline{\underline{\tau}} - \tau_n = \underline{\underline{\tau}} - (\tau_n) \cdot \vec{n} \cdot \vec{n}$$

- Hidrostatsično napetostno stanje $\underline{\underline{t}} = -p\underline{\underline{I}}$; v vsaki smeri je normalna napetost enaka $-p$, strižna napetost je enaka nič.

- Strižno napetostno stanje, obstaja KS v katerem je vsota diagonalnih elementov napetostnega tenzorja enaka nič. Izračun normalne in strižne napetosti.

- Ravninsko napetostno stanje.

